

## C cross section tension verification using S16-19:

### 1.1.1 Description

The test verifies the tension strength of C shape section column.

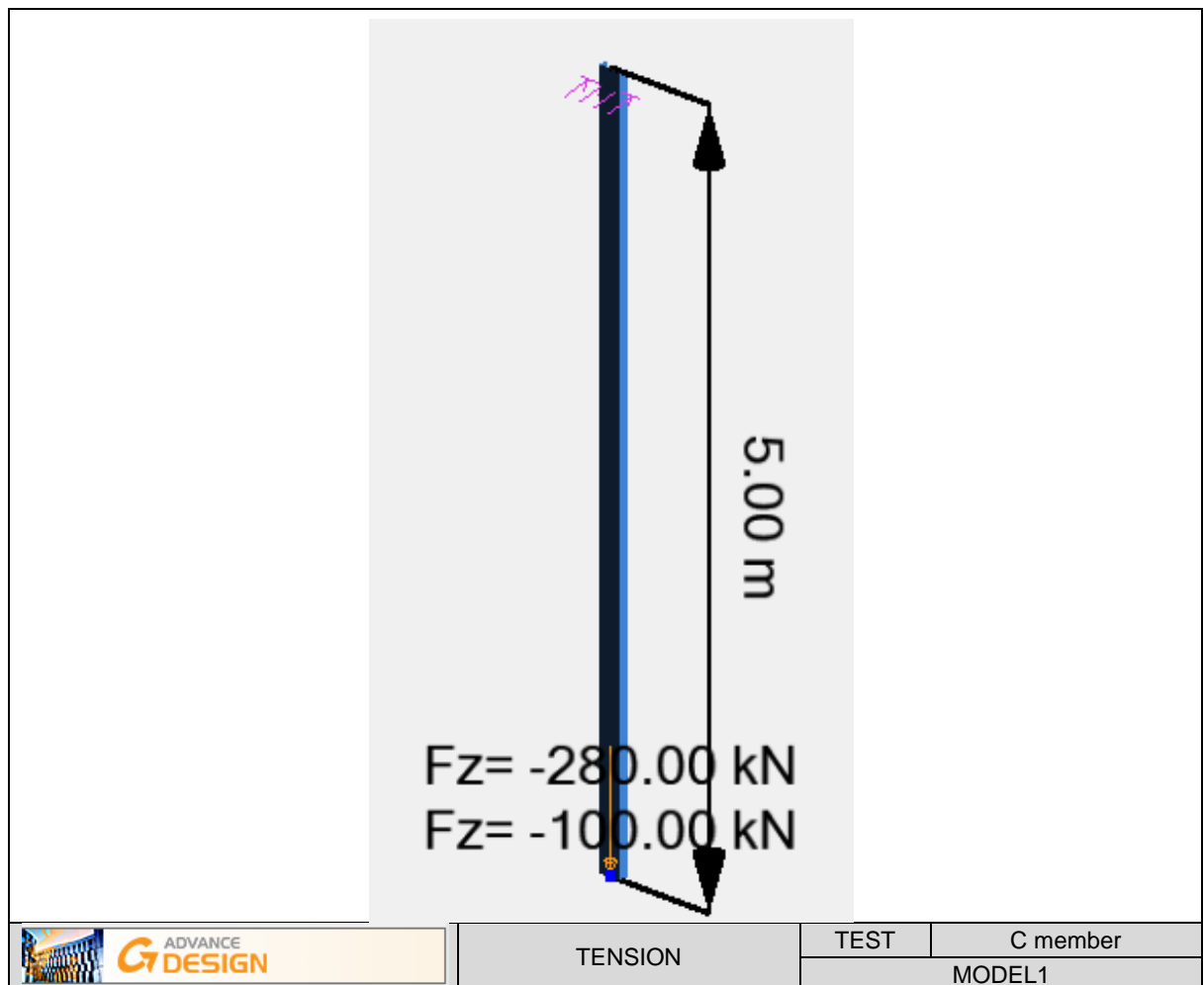
The column is subjected to end tension.

### 1.1.2 Background

The test verifies the available tension strength of a C180x22 column shown in the figure below. The column is subjected to tension of 100 kN from dead load and 280 kN from live load. Material G40.21M-350W ( $F_y=350$  MPa) steel is selected for this example.

#### 1.1.2.1 Model description

- Analysis type: static linear (plane problem)
- Element type: linear
- The following load case is used:
- Load cases:  $F_D = -100$  kN ;  $F_L = -280$  kN

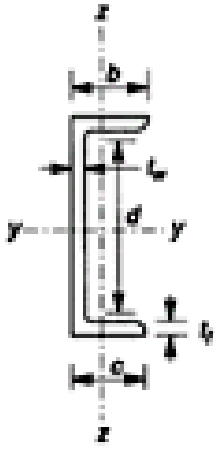


#### Units

Metric System

Geometry

- Cross section:

<u>Dimensions and surface area</u>		<u>Properties</u>		
	<b>Area</b>		<b>Strong Axis (Y-Y)</b>	
	<b>A</b>	27.90 cm <sup>2</sup>	<b>I<sub>y</sub></b>	1130 cm <sup>4</sup>
	<b>Depth</b>		<b>S<sub>y</sub></b>	127 cm <sup>3</sup>
	<b>h</b>	17.78 cm	<b>Z<sub>y</sub></b>	163 cm <sup>3</sup>
	<b>Web thickness</b>		<b>Weak Axis (Z-Z)</b>	
	<b>t<sub>w</sub></b>	1.06 cm	<b>I<sub>z</sub></b>	57 cm <sup>4</sup>
	<b>Flange width</b>		<b>S<sub>z</sub></b>	47.4 cm <sup>3</sup>
	<b>b</b>	5.80 cm	<b>Z<sub>z</sub></b>	26.7 cm <sup>3</sup>
	<b>Flange thickness</b>		<b>Shear area</b>	
	<b>t<sub>f</sub></b>	0.93 cm	<b>A<sub>y</sub></b>	10.79 cm <sup>2</sup>
	<b>Fillet radius</b>		<b>A<sub>z</sub></b>	18.87 cm <sup>2</sup>
<b>r</b>	0.99 cm	<b>Torsional constant</b>		
		<b>J</b>	13.92 cm <sup>4</sup>	
		<b>Warping constant</b>		
		<b>C<sub>w</sub></b>	3435.72 cm <sup>6</sup>	
<b><u>C180x22</u></b>				

- Beam length: L =500 cm

Materials properties

Steel G40.21M-350W is used. The following characteristics are used in relation to this material:

- Yield strength  $f_y=350$  MPa
- Longitudinal elastic modulus:  $E=200000$  MPa
- Shear modulus of rigidity:  $G=76923.1$  MPa

Boundary conditions

The boundary conditions are described below:

- Outer:
  - ▶ Support at X = 500 cm (Restrains: TX, TY, TZ, Rx, Ry, Rz)
- Inner: None.

Loading

The column is subjected to the following load combinations and actions:

- ULS:  $q = 1.25 \times D + 1.5 \times L$
- LSS:  $q = 1 \times D + 1 \times L$

### 1.1.2.2 Reference results in calculating

#### Reference solution

From the NBC 2015, the required tension strength for the design is:

ULS
$ Fx  = 1.25 \times 100 + 1.5 \times 280$ $ Fx  = 545 \text{ kN}$

The Factored tensile resistance  $T_r$  of a member subjected to axial tension is computed from the clause 13.2:

$$T_r = \phi \times A_g \times F_y$$

In order to verify the plasticity of the gross section with the factored load computed at ULS:

ULS
$\phi = 0.9$ $T_r = \phi \times A_g \times F_y = 0.9 \times 2790 \times 350$ $= 878.85 \text{ kN}$ $T_f = Fx = 545 \text{ kN} < T_r = 878.85 \text{ kN} \quad \text{O.K}$ Work ratio: $r = \frac{T_f}{T_r} = 62\%$

Since the member is under tension forces only the verification of bending resistance and combined forces is unnecessary.

#### Finite elements modeling


- Linear element: S beam,
- 6 nodes,
- 1 linear element.

### 1.1.2.3 Results comparison

Result name	Result description	Reference value	AD value	Percent Difference
$T_r$	Factored tensile resistance	878.85 kN	897.75 kN	0.00%
r	Design ratio	62%	62%	0.00%

	Unfavorable case	Verification	Work ratio
Tension Compression	n°102	$C_f \leq C_r$ (13.2) 545.00 < 878.85 kN	62%
Combined forces	n°102	$T_f/Tr + M_f/Mr \leq 1$ (13.9.1) 0.620 < 1.000	62%

	TENSION	TEST	C section
		RESULTS 1	



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